



Innovation and **N**etworking for **F**atigue and  
**R**eliability **A**nalysis of **S**tructures – **T**raining for  
**A**sessment of **R**isk



# Structural and action models : The benefit of monitoring

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ÉCOLE POLYTECHNIQUE  
FÉDÉRALE DE LAUSANNE



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10 October 2018

2<sup>nd</sup> Implementation Day, Paris – Vélizy Villacoublay, France

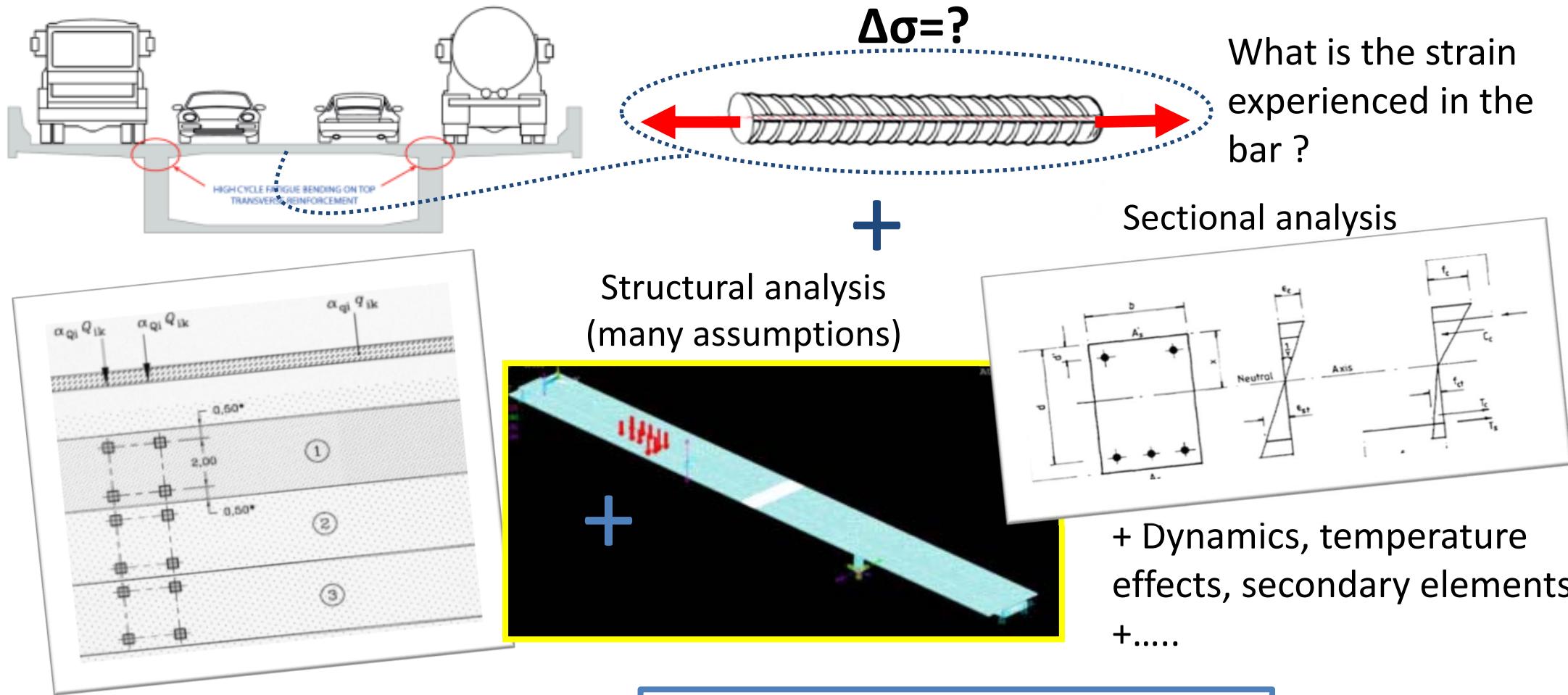
# Motivation

- High loss of revenue and productivity due to **inefficient infrastructure**
    - *bridges and WT structures are the critical link*
  - Resistance models are significantly advanced but there is still much uncertainty associated with the **loading conditions** safety problem
  - While satisfactory for new structures, they are often **overly conservative** for existing ones
  - Low cost sensors allow action effects to be monitored easily
  - New structural engineering techniques required for safety verification based on **monitored data**
- Our task is to show that there is no fatigue problem !**



# Why monitoring of traffic action effect on structural members ?

Traditional approach for action effect determination:

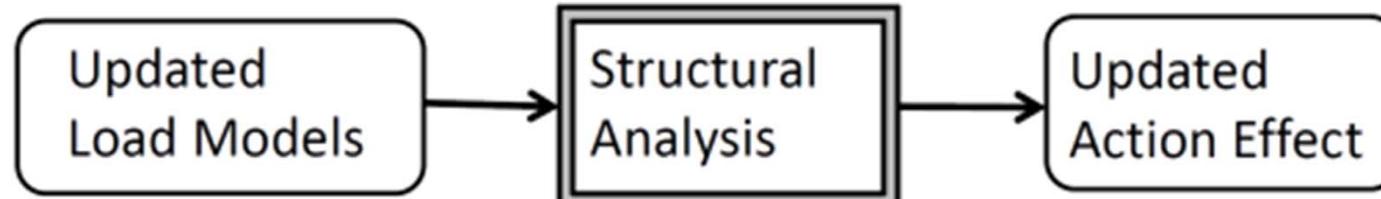


Updated load models

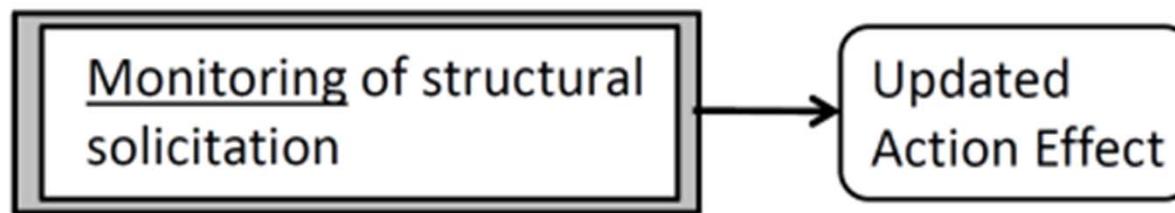
These effects are inherent  
in the monitoring result!

# Determination of updated action effect following a stepwise procedure : *The role of structural models*

Level 1 :

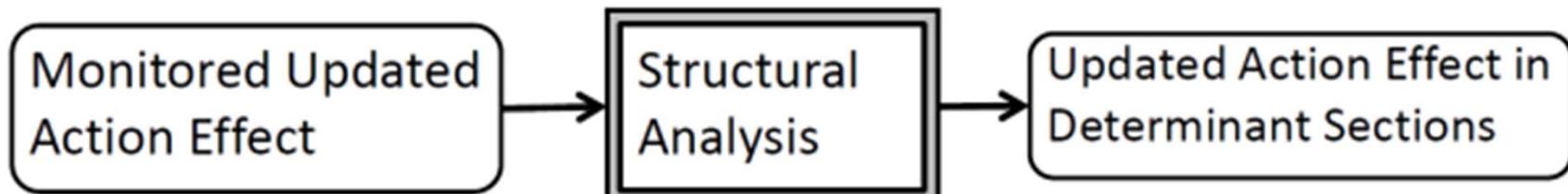


Level 2 :



Monitoring reduces uncertainties.

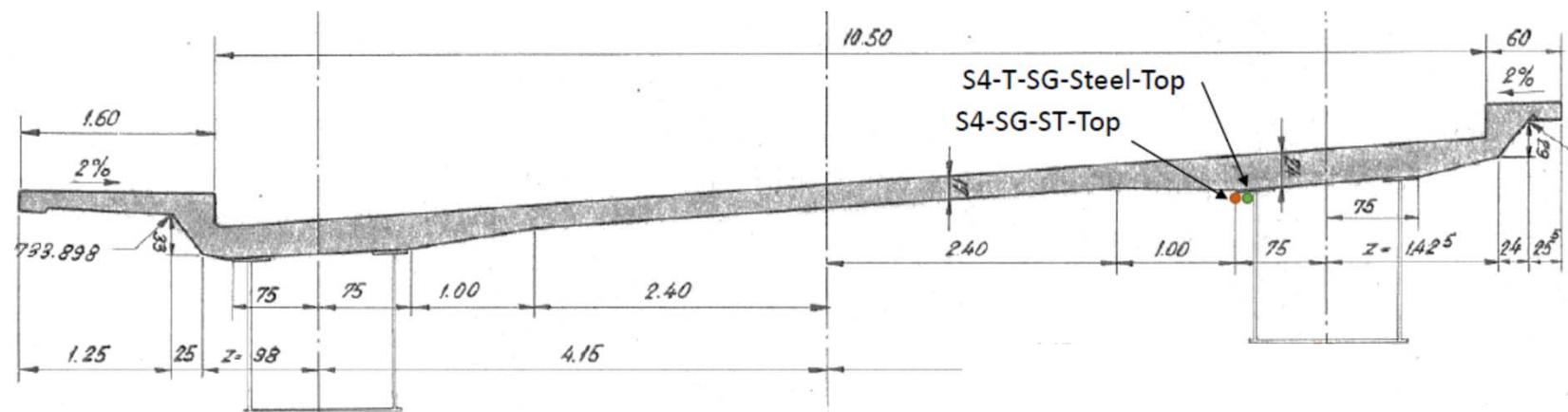
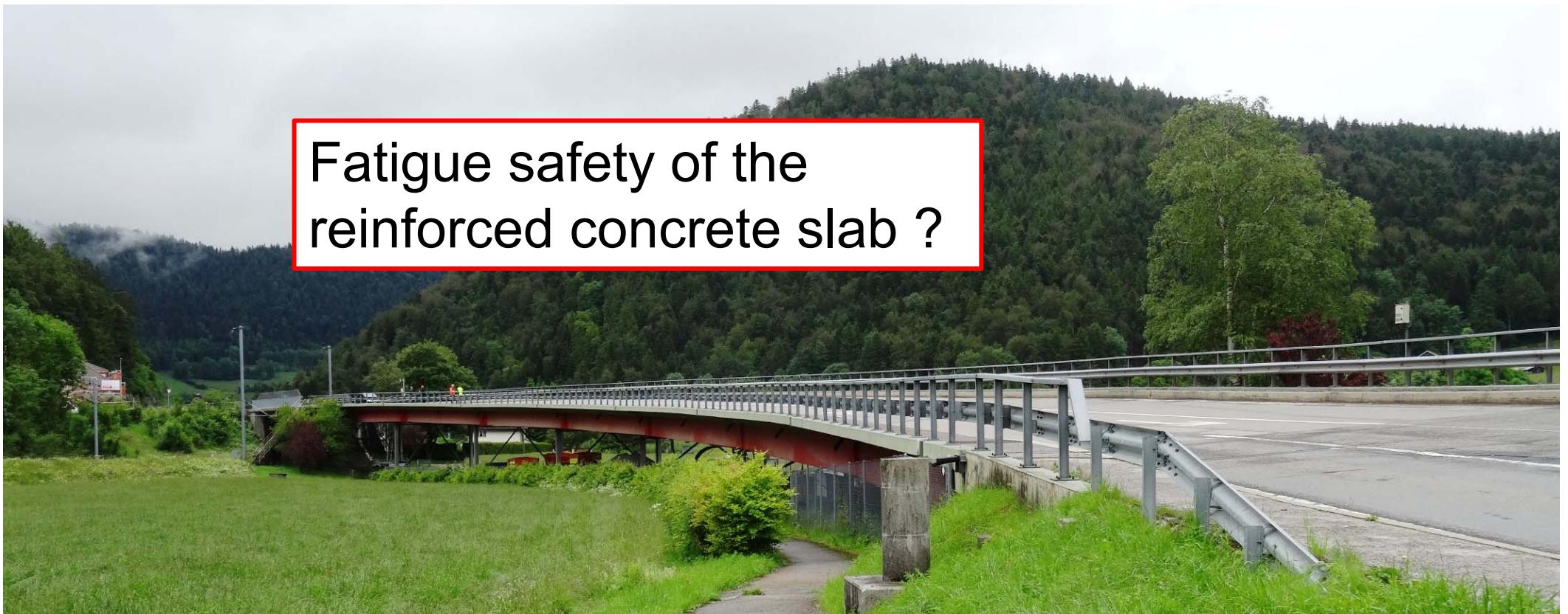
Effect of permanent actions can not be monitored.



# «Crêt de l'Anneau» Viaduct, 1958

Composite steel – concrete structure

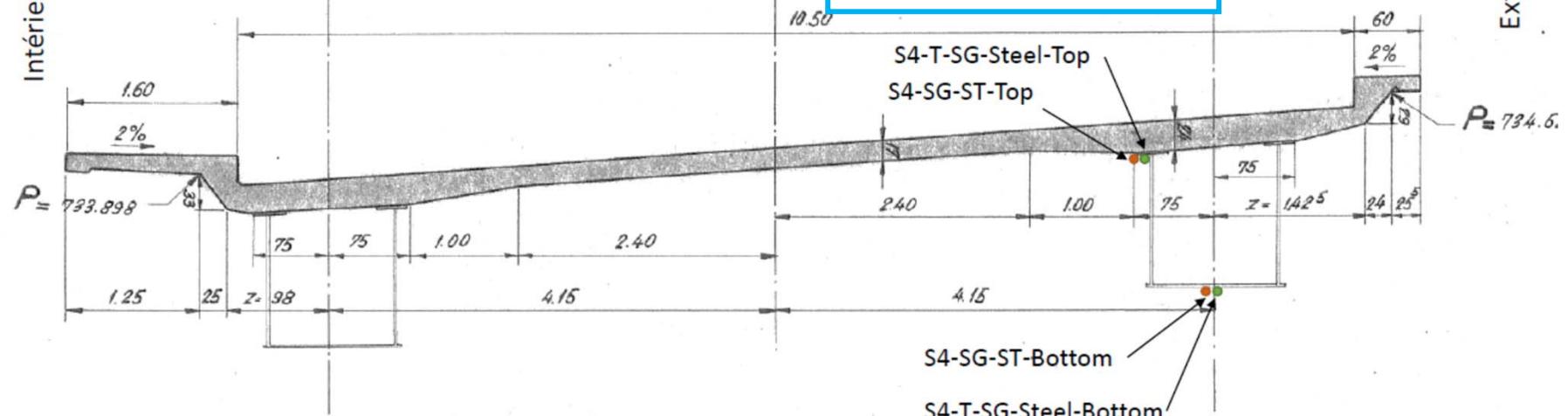
Fatigue safety of the reinforced concrete slab ?



- Jauge en ¼ de pont – 4 fils
- Thermocouple

# Monitoring: sensor deployment

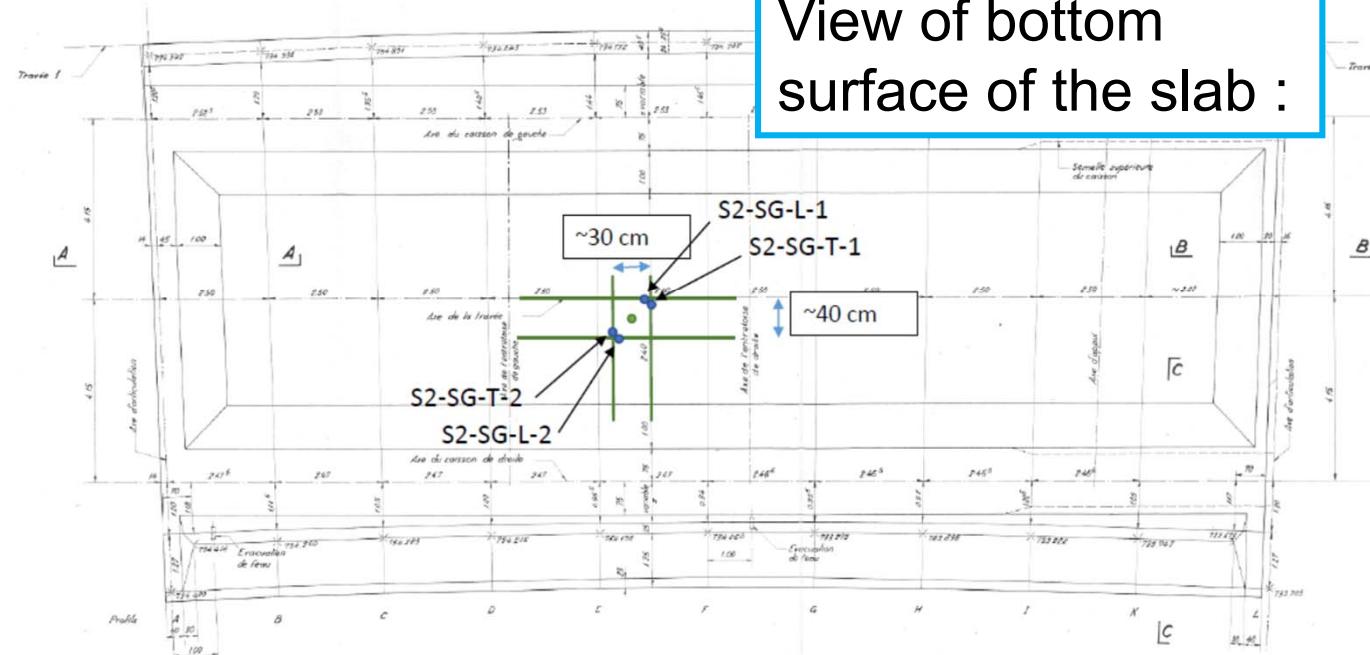
Intérieur



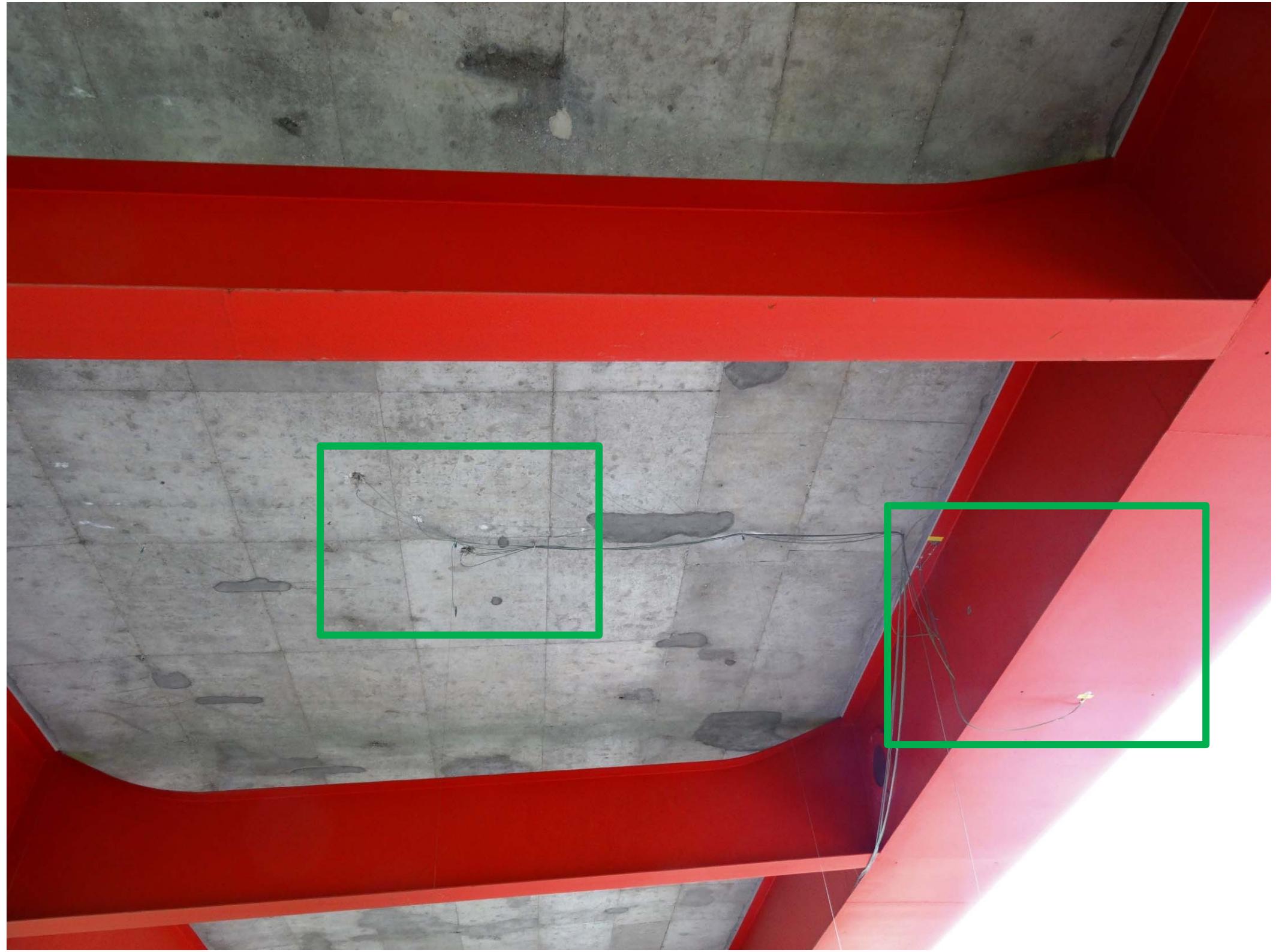
cross section :

- Jauge en ½ de pont – 5 fils
- Thermocouple

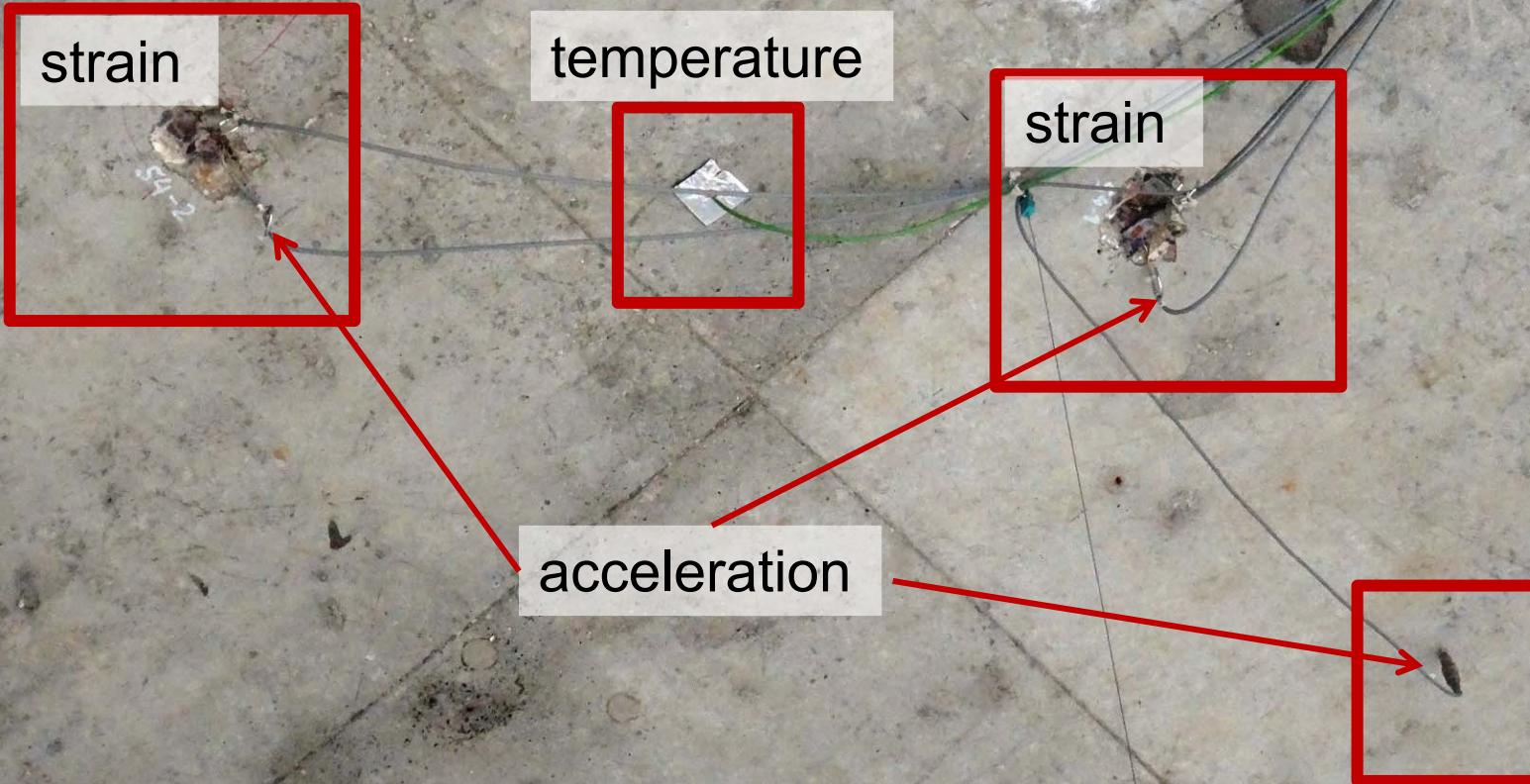
View of bottom surface of the slab :



Extérieur



## Pocket monitoring



In addition: deflection measurements

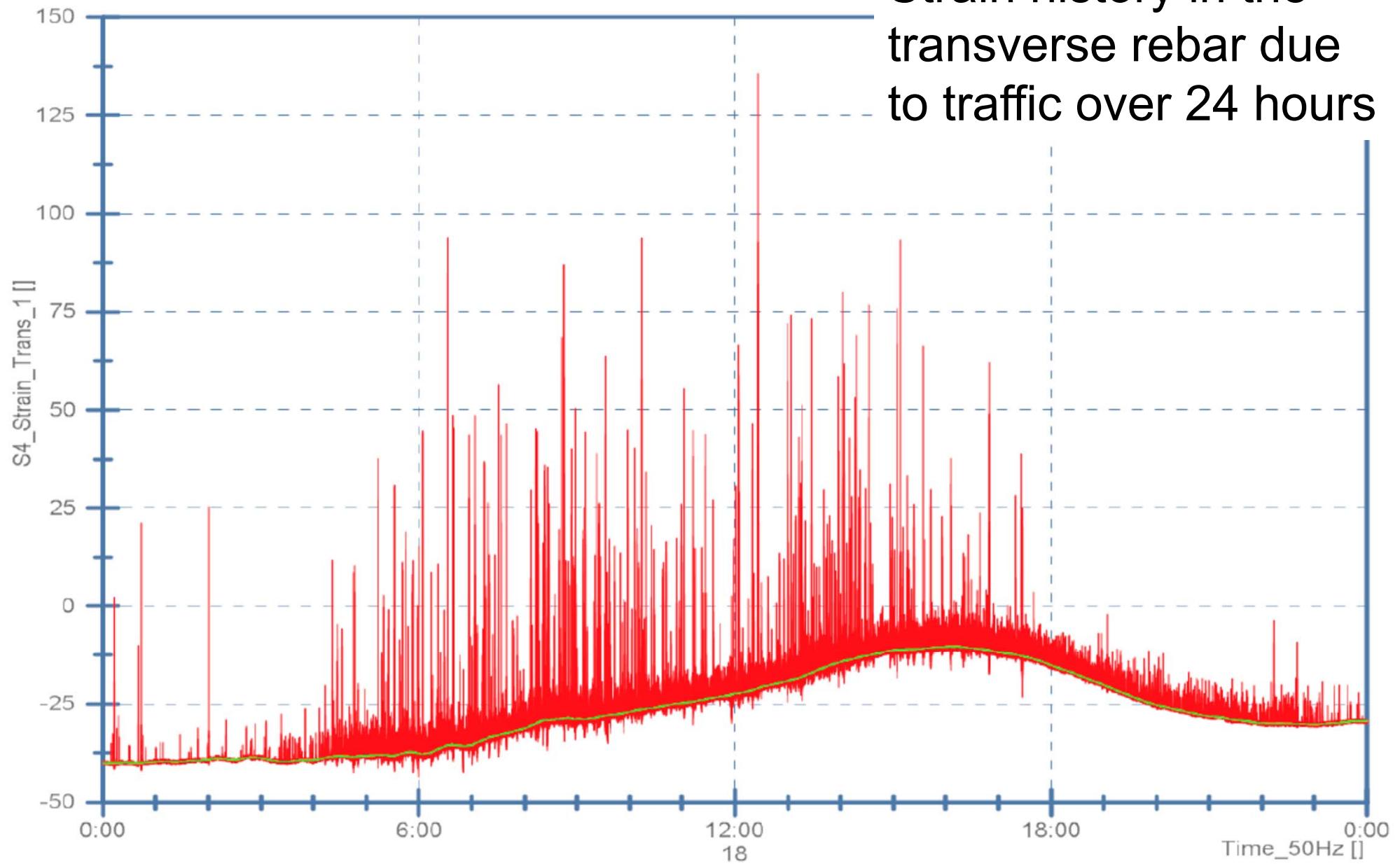
Charges par essieu [kg] (total 40'030 kg)



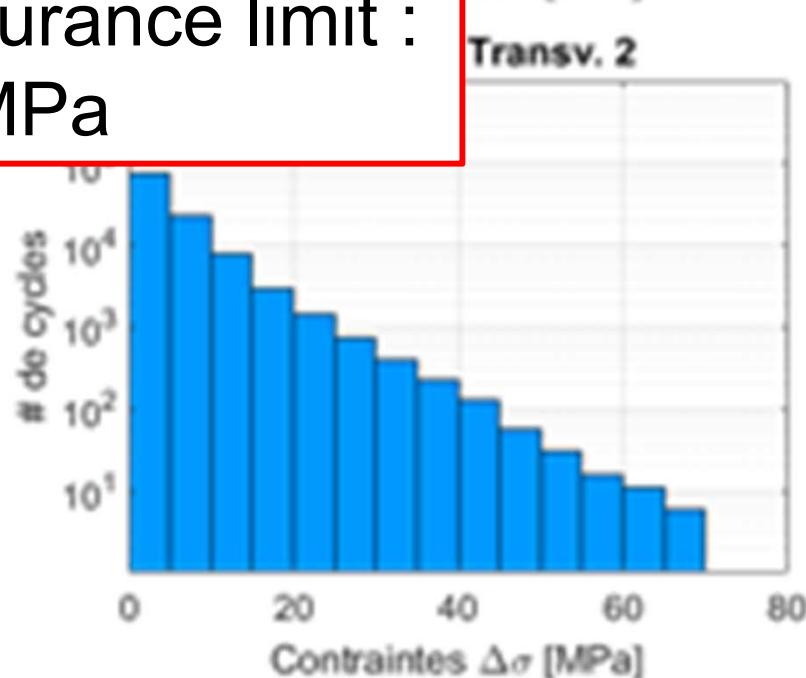
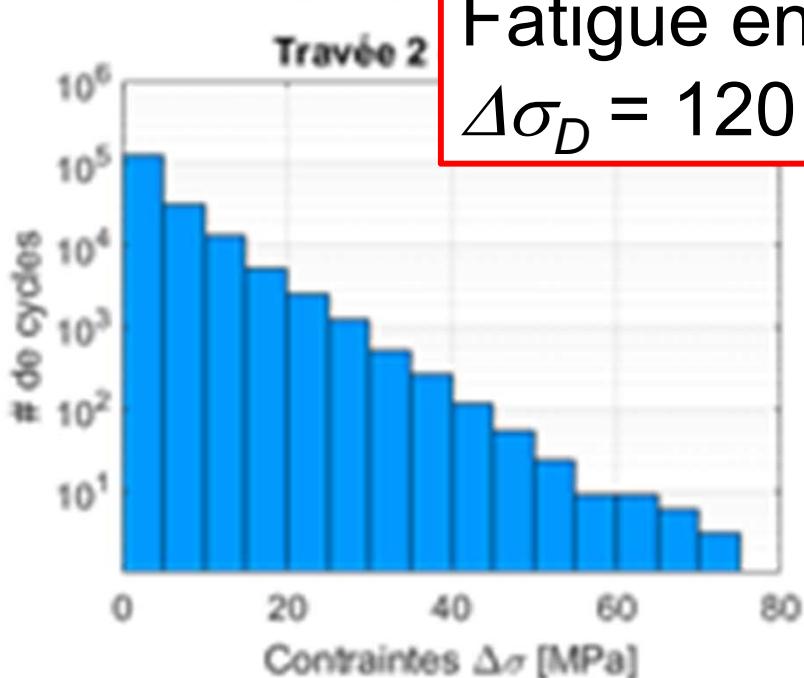
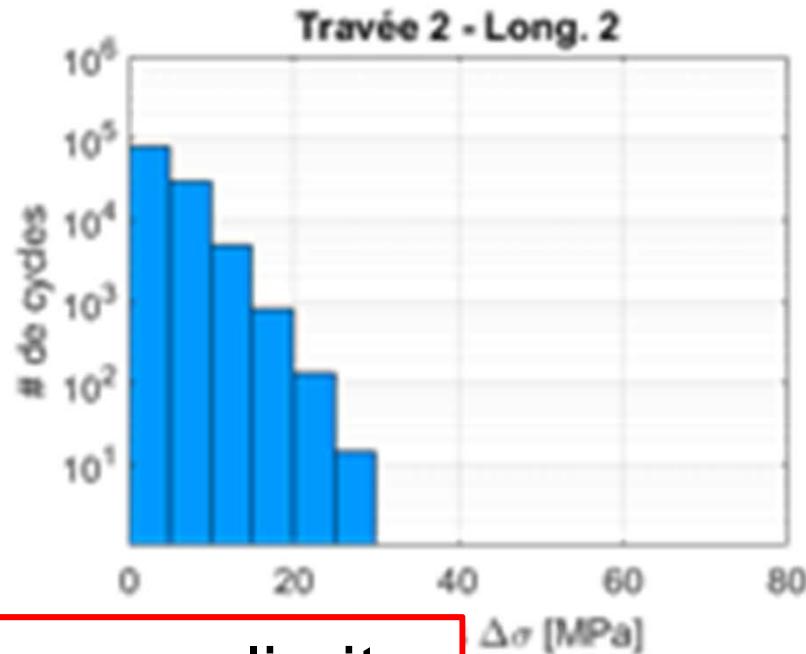
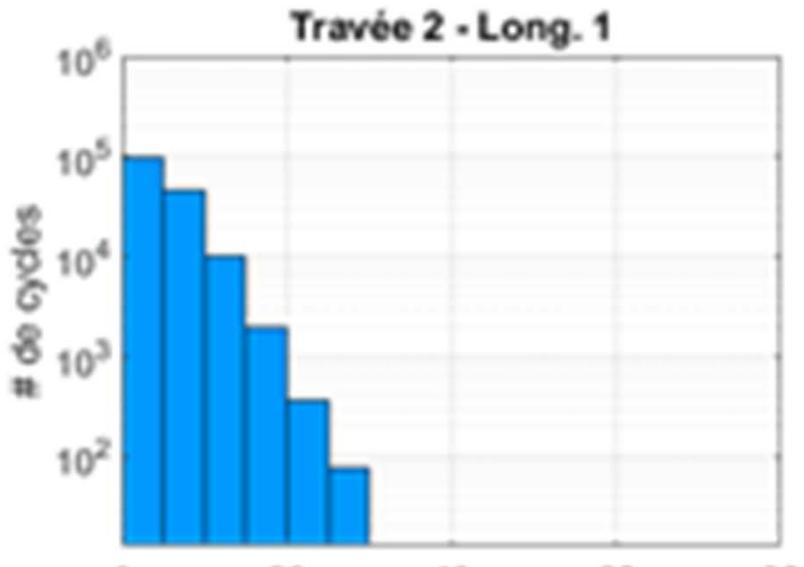
## Testing with a truck of known weight



Strain history in the  
transverse rebar due  
to traffic over 24 hours



# Annual histograms of tensile stress amplitudes for the four instrumented rebars at one slab zone

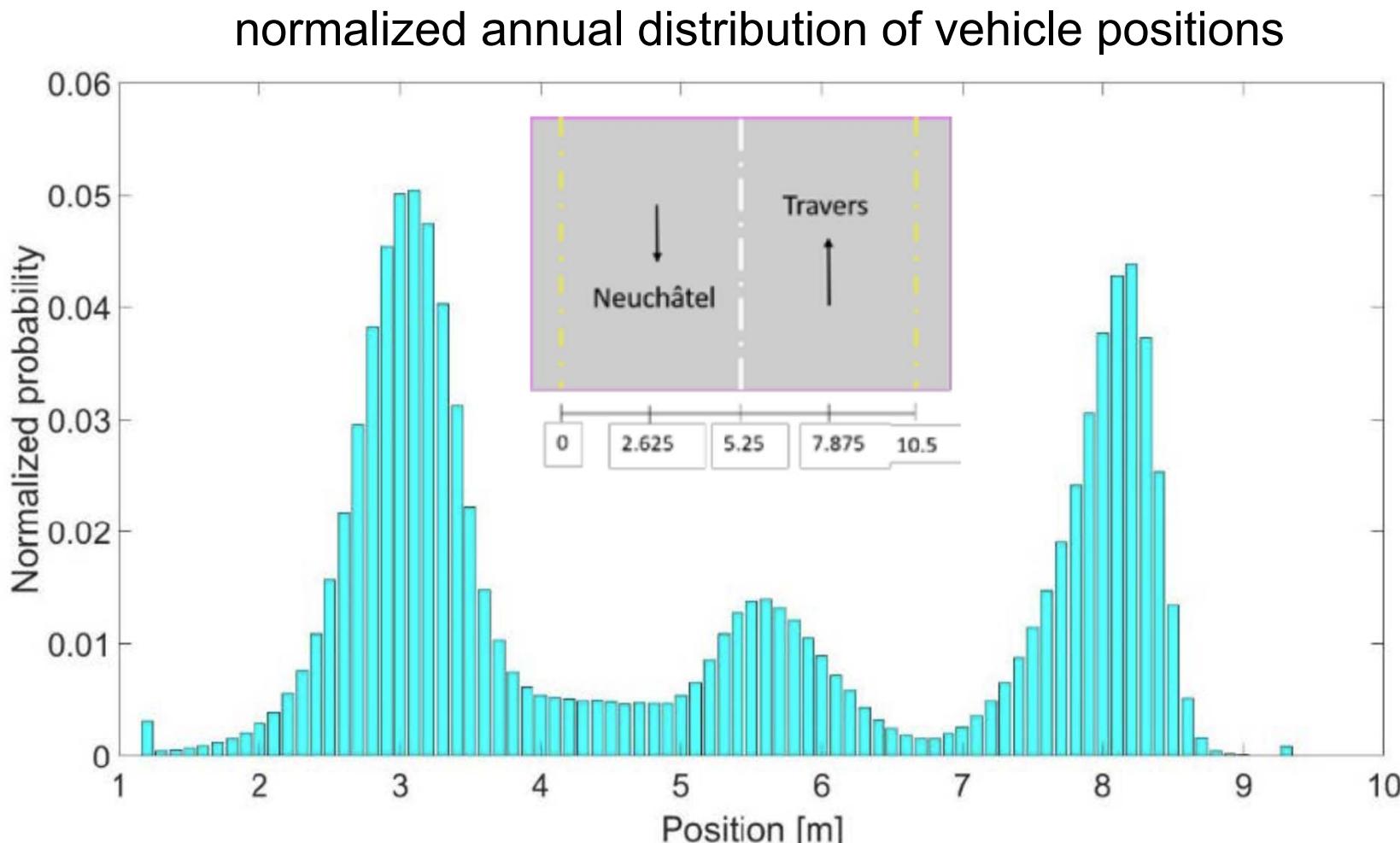


Fatigue endurance limit :  
 $\Delta\sigma_D = 120 \text{ MPa}$

# Quantification of road traffic and temperature effects on fatigue of a RC bridge deck based on data from monitoring

Authors: Imane Bayane <sup>(1)</sup>, Amol Mankar <sup>(2)</sup>, Eugen Brühwiler <sup>(1)</sup>, John Dalsgaard Sørensen <sup>(2)</sup>

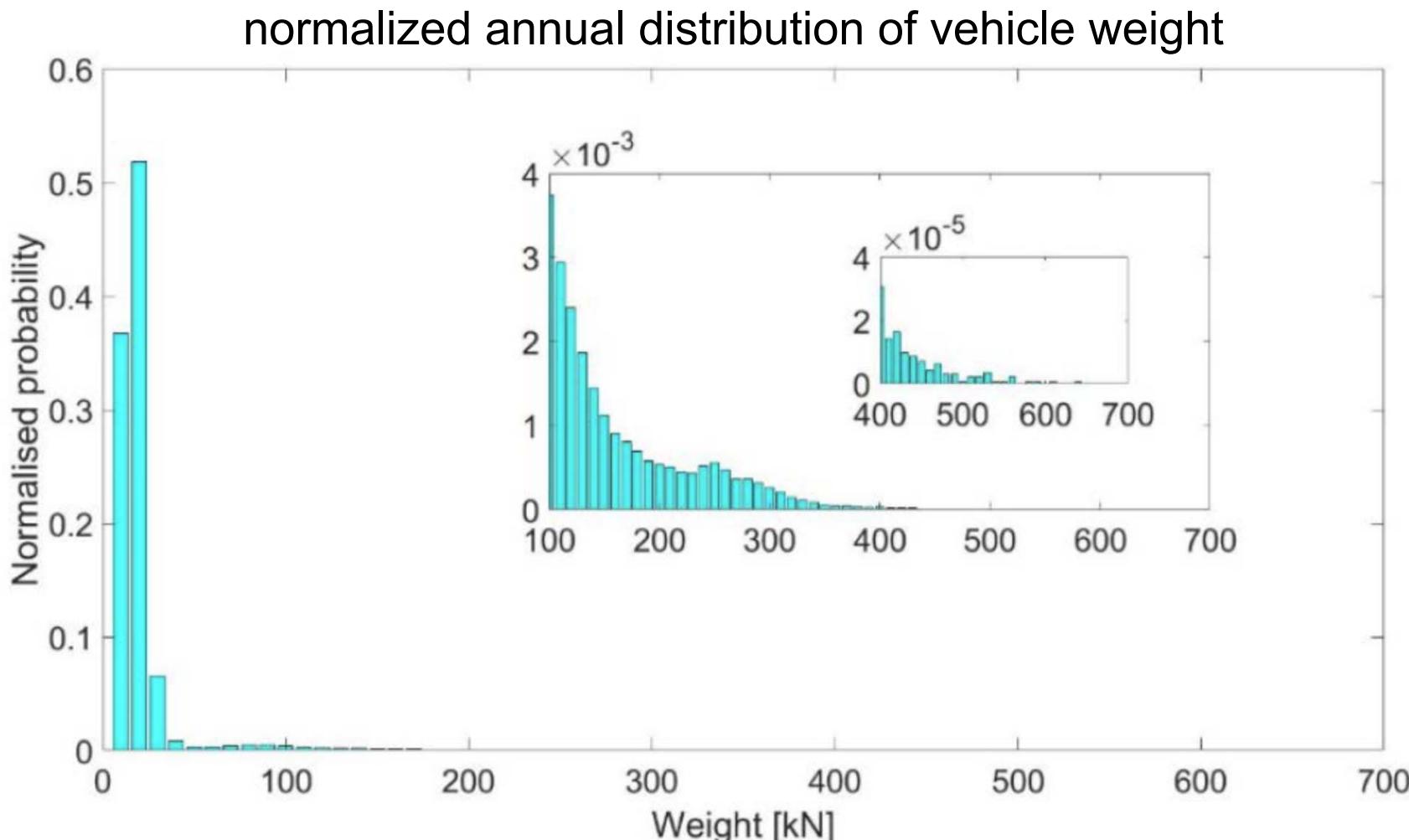
1. EPFL ENAC IIC MCS, Ecole Polytechnique Fédérale de Lausanne
2. Aalborg University, Thomas Manns Vej, 23, 9220 Aalborg.



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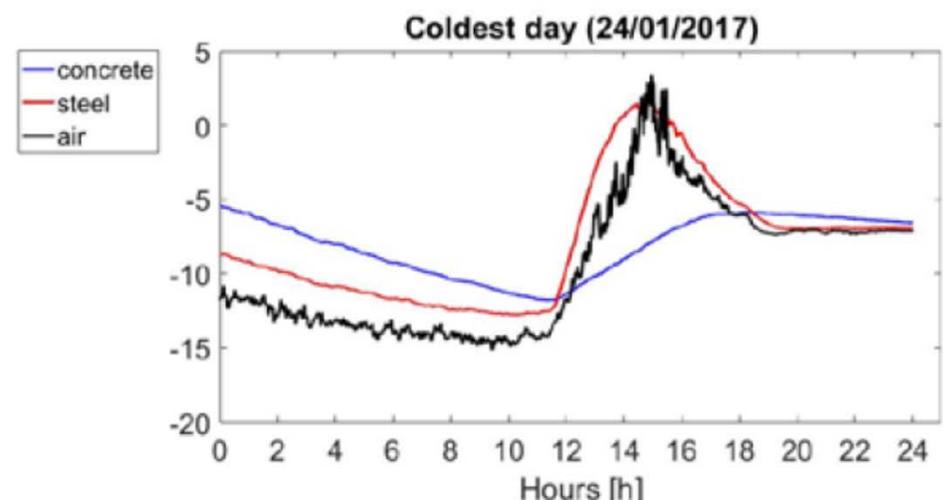
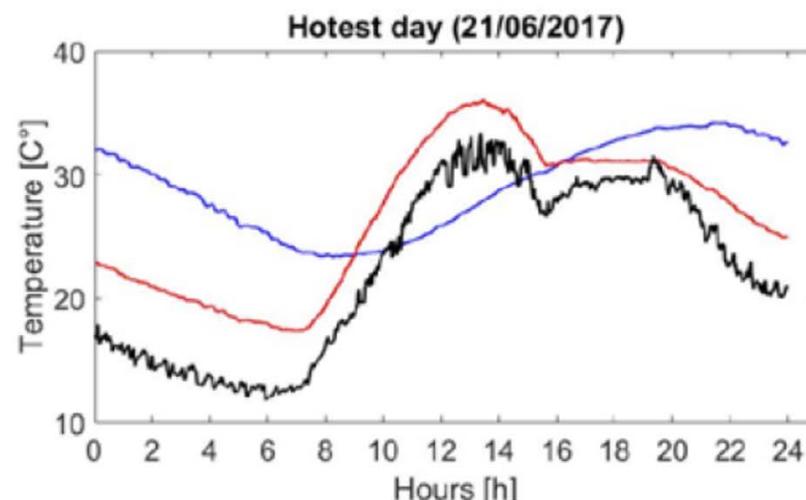
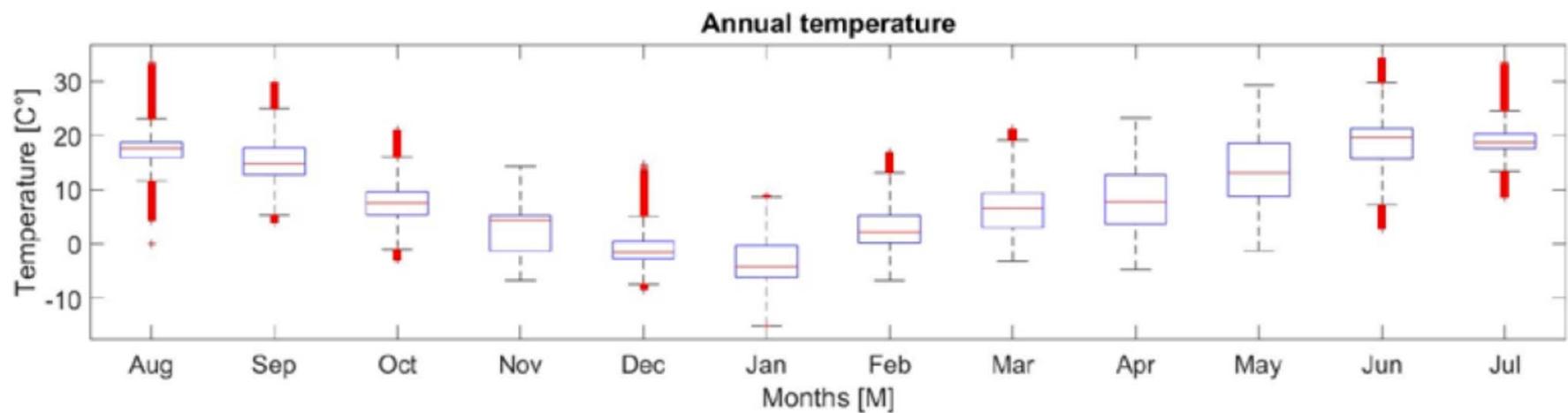
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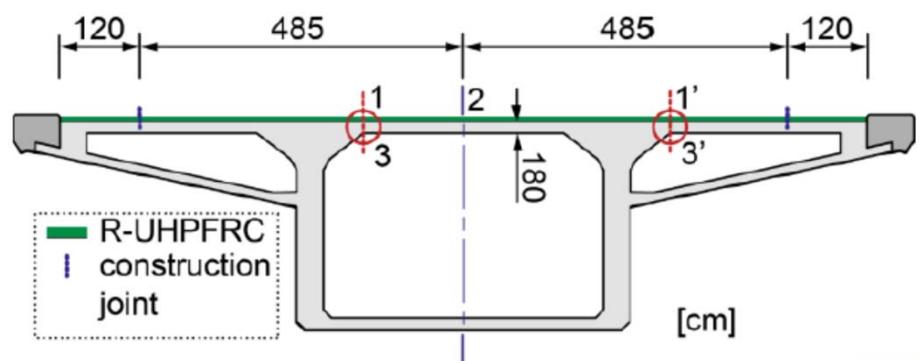
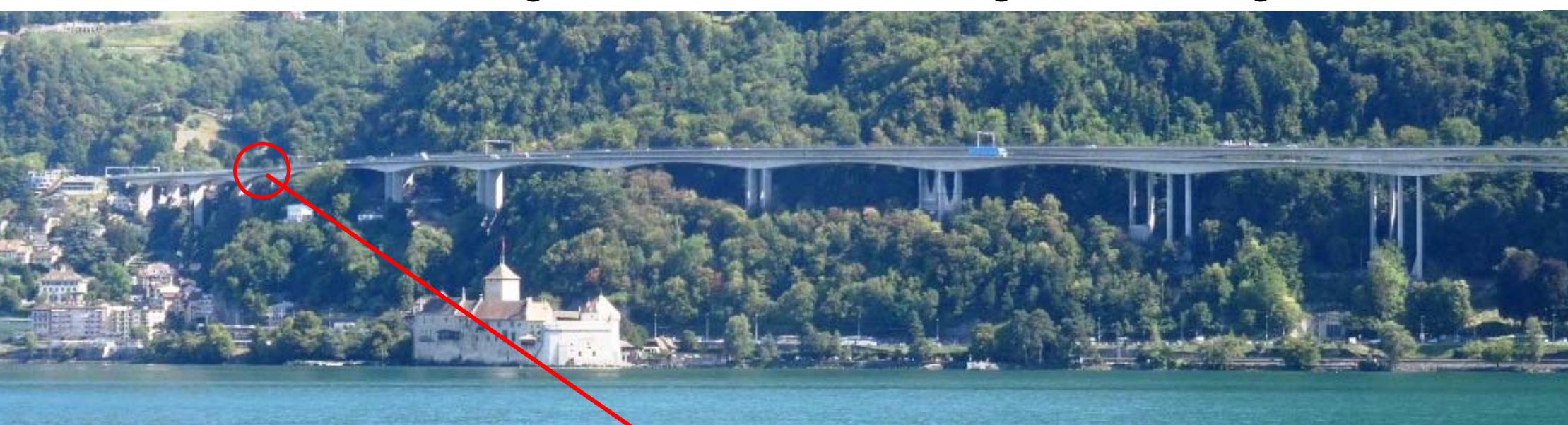
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# Chillon Viaduct: Monitoring of the deck slab strengthened using R-UHPFRC



Measurement of :

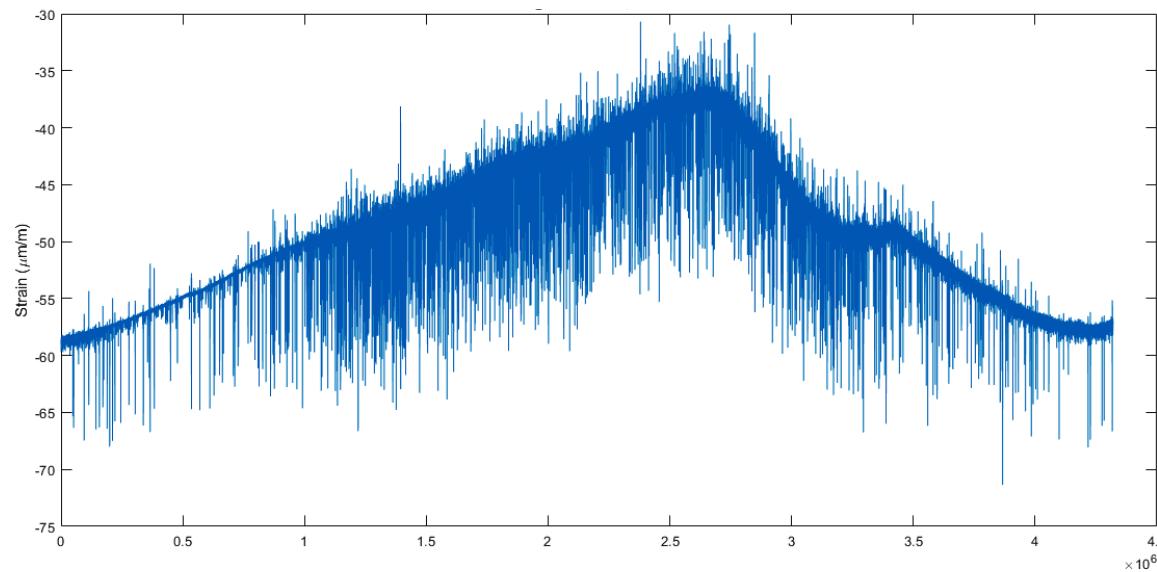
- rebar strain
- temperature
- accelerations (ETH Zurich)  
for system identification



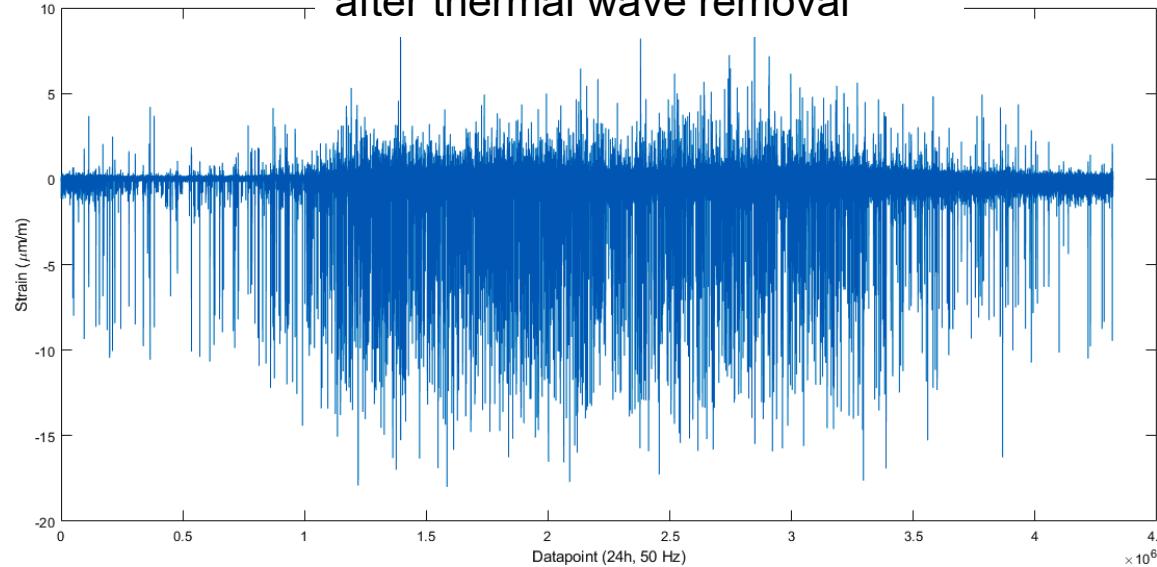
Interview with Swiss Radio about Monitoring  
of Civil Structures, 25 Sept. 2018

# Chillon Viaduct: Monitoring of the deck slab strengthened using R-UHPFRC

31 Aug 2016: longitudinal rebar

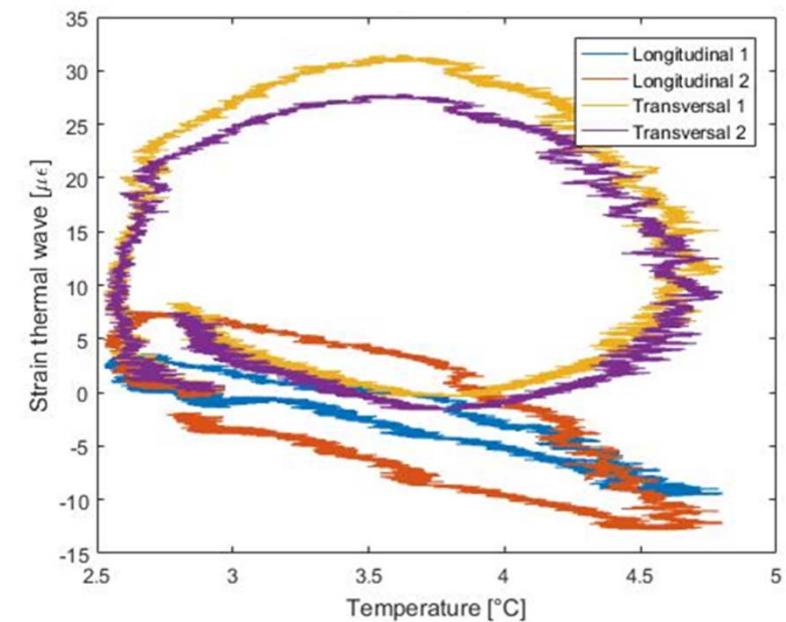


31 Aug 2016: longitudinal rebar  
after thermal wave removal



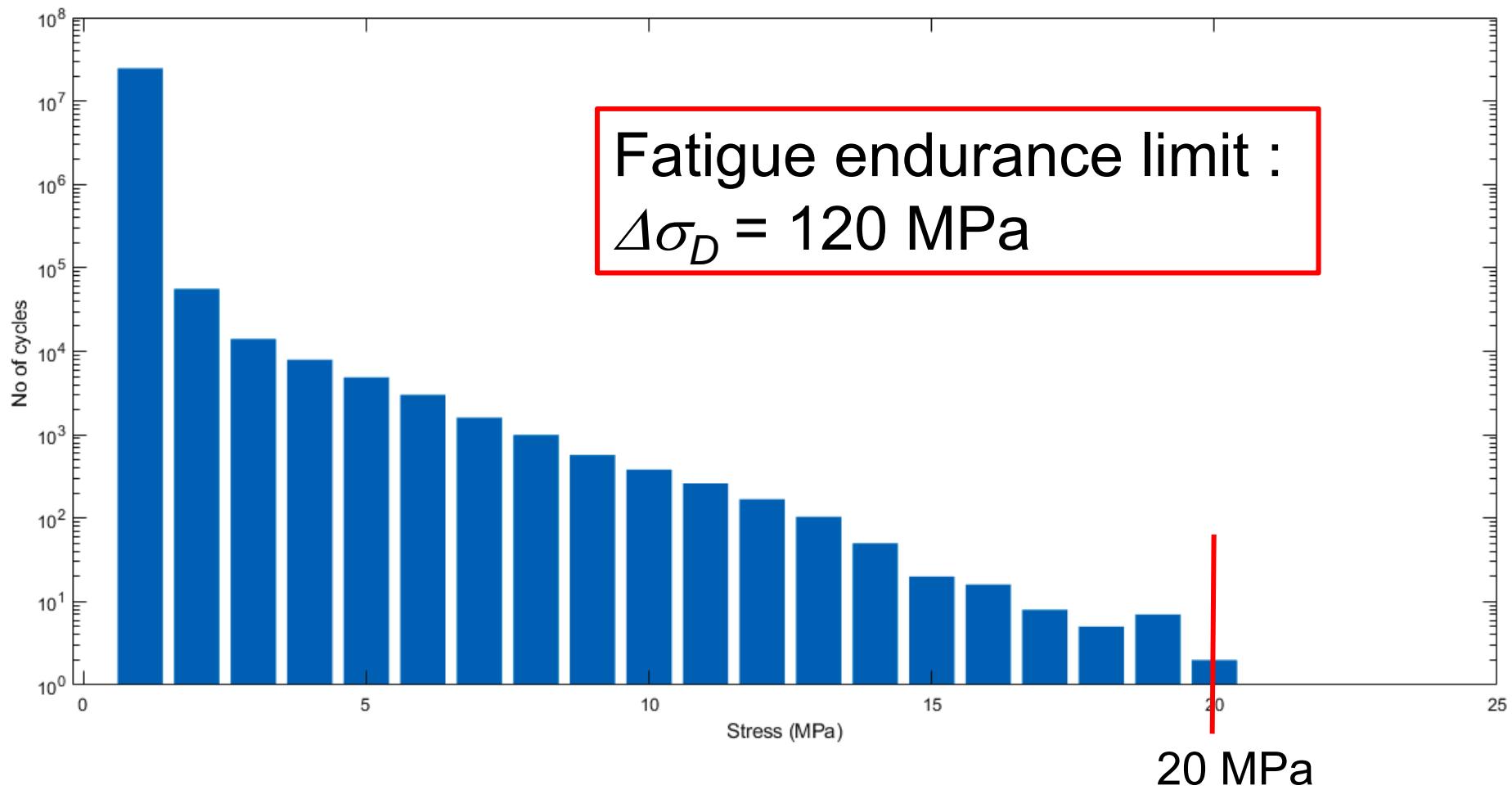
Analysis of measurements by  
Bartek Sawicki and Morteza  
Ahmadivala : joint paper

15 Nov 2017: «thermal wave»



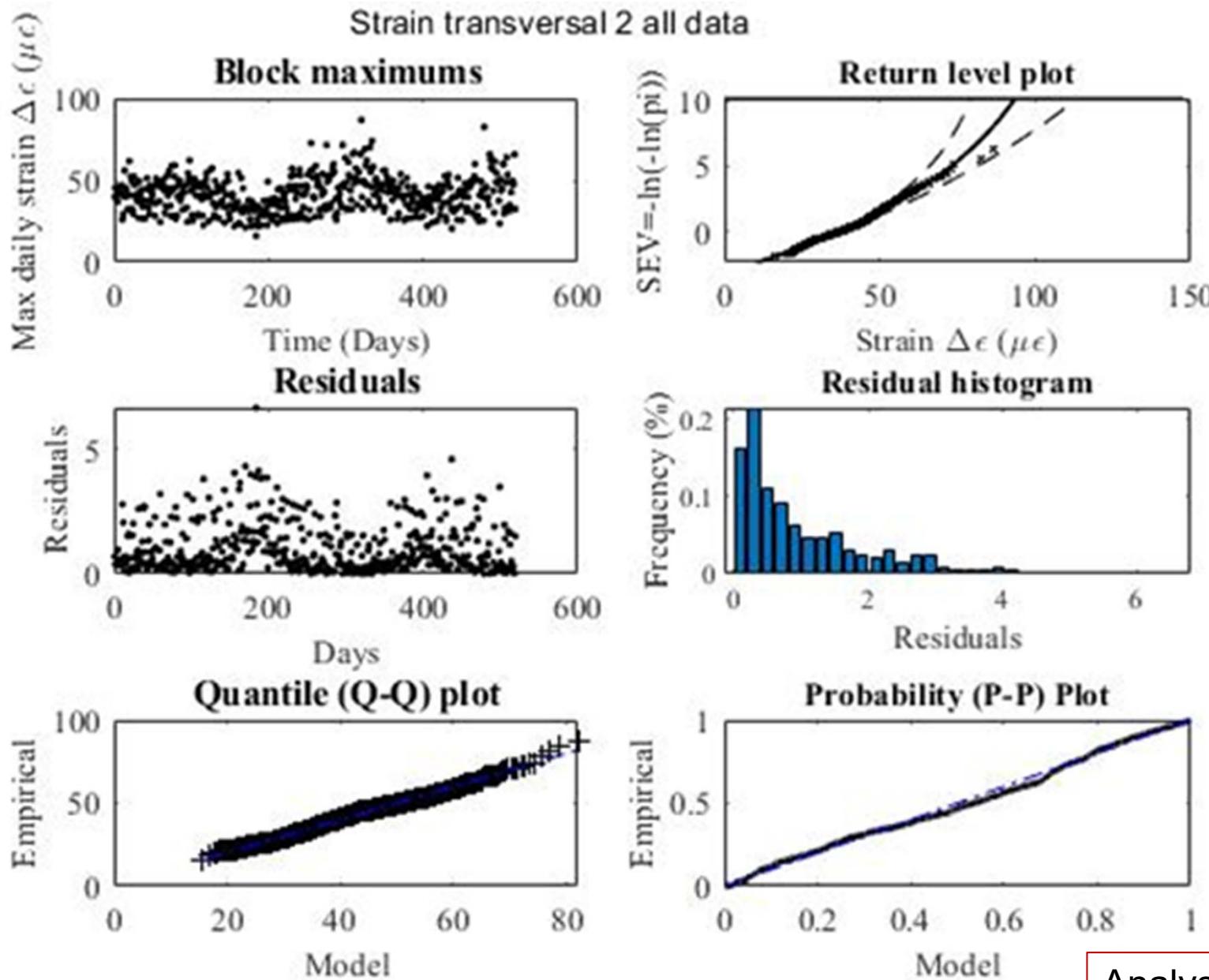
# Chillon Viaduct: Monitoring of the deck slab strengthened using R-UHPFRC

Histogram of tensile stress amplitudes in the transverse rebar (August 2016, 31 days)



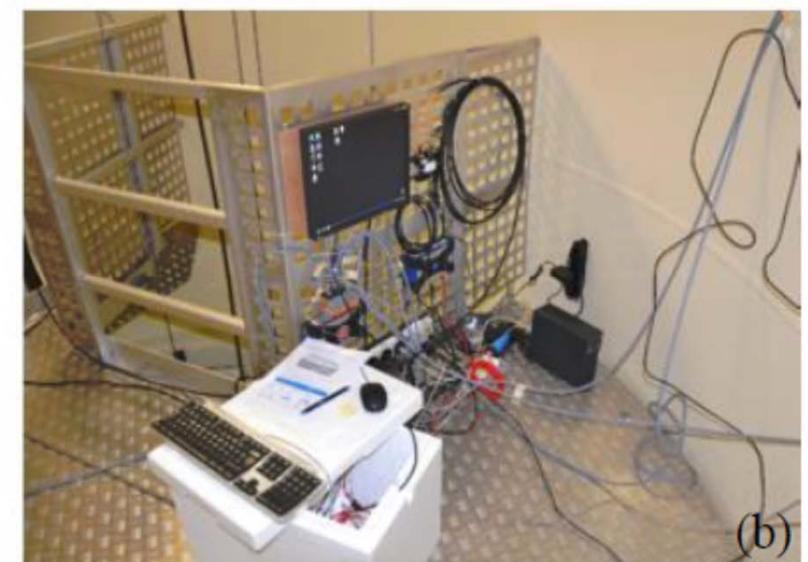
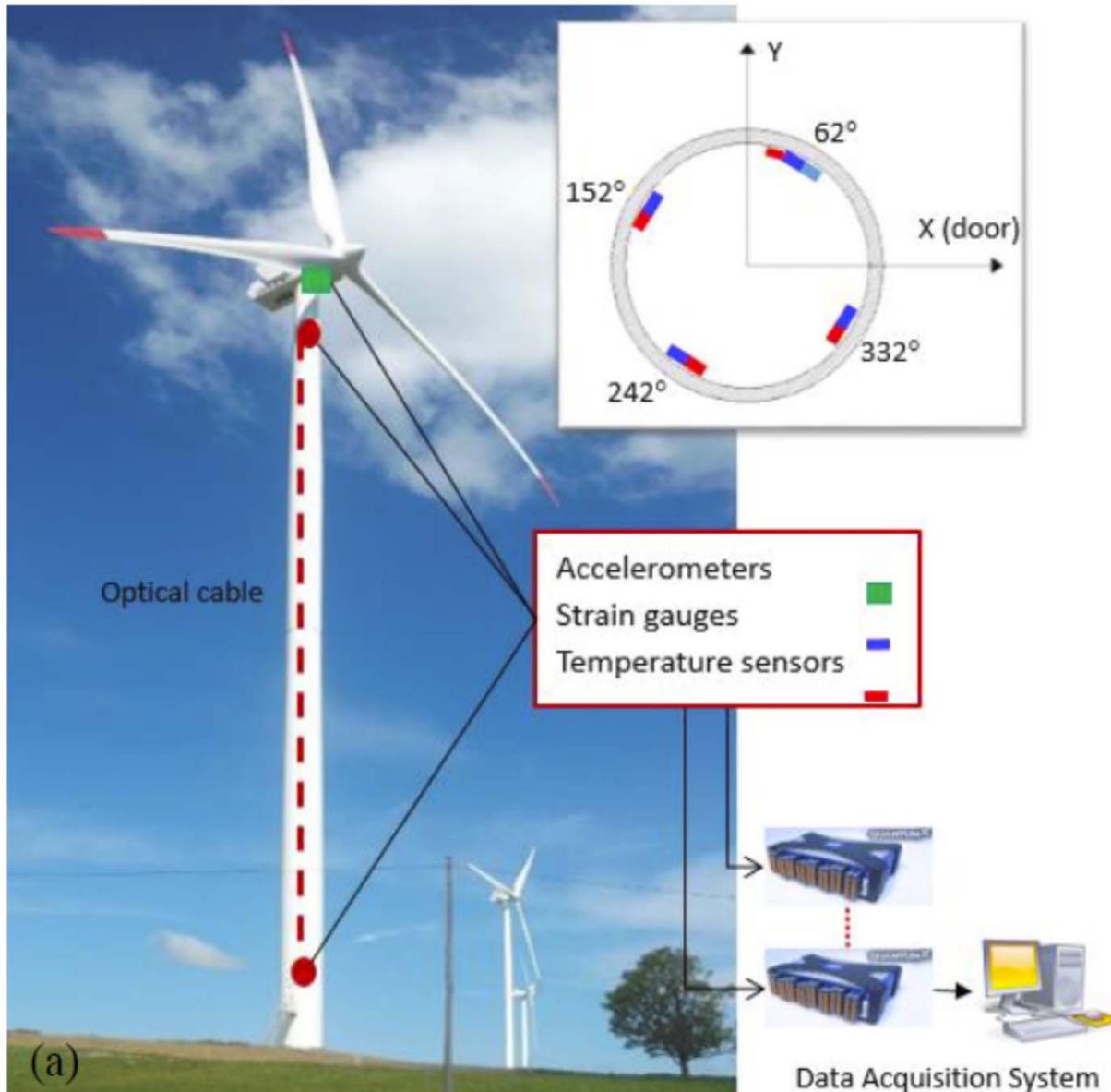
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# Chillon Viaduct: Monitoring of the deck slab strengthened using R-UHPFRC



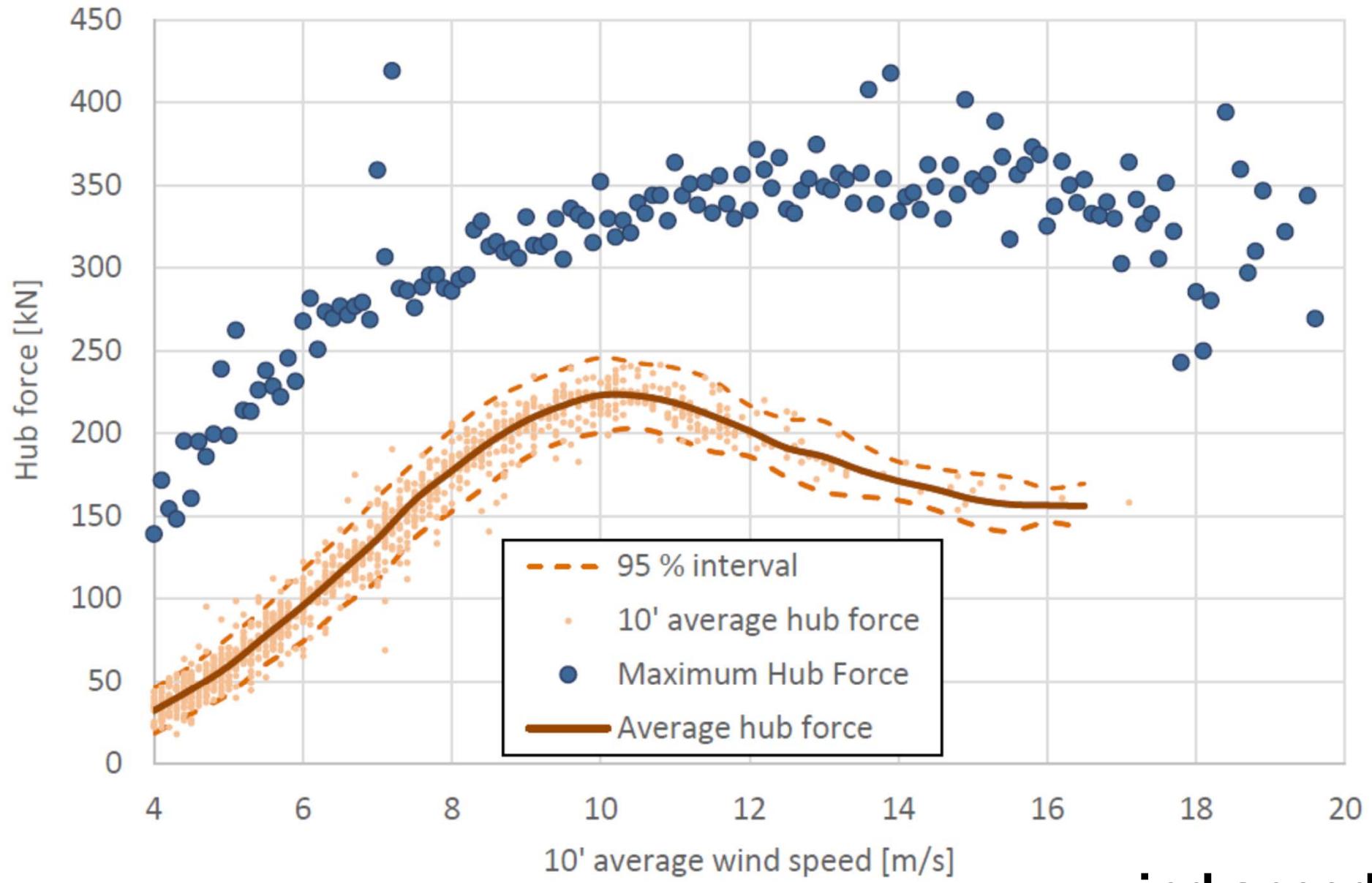
Analysis of measurements by  
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Ahmadivala : joint paper

# Fatigue action effect on wind turbine tower : *Data from monitoring*



[Loraux, Brühwiler, 2016]

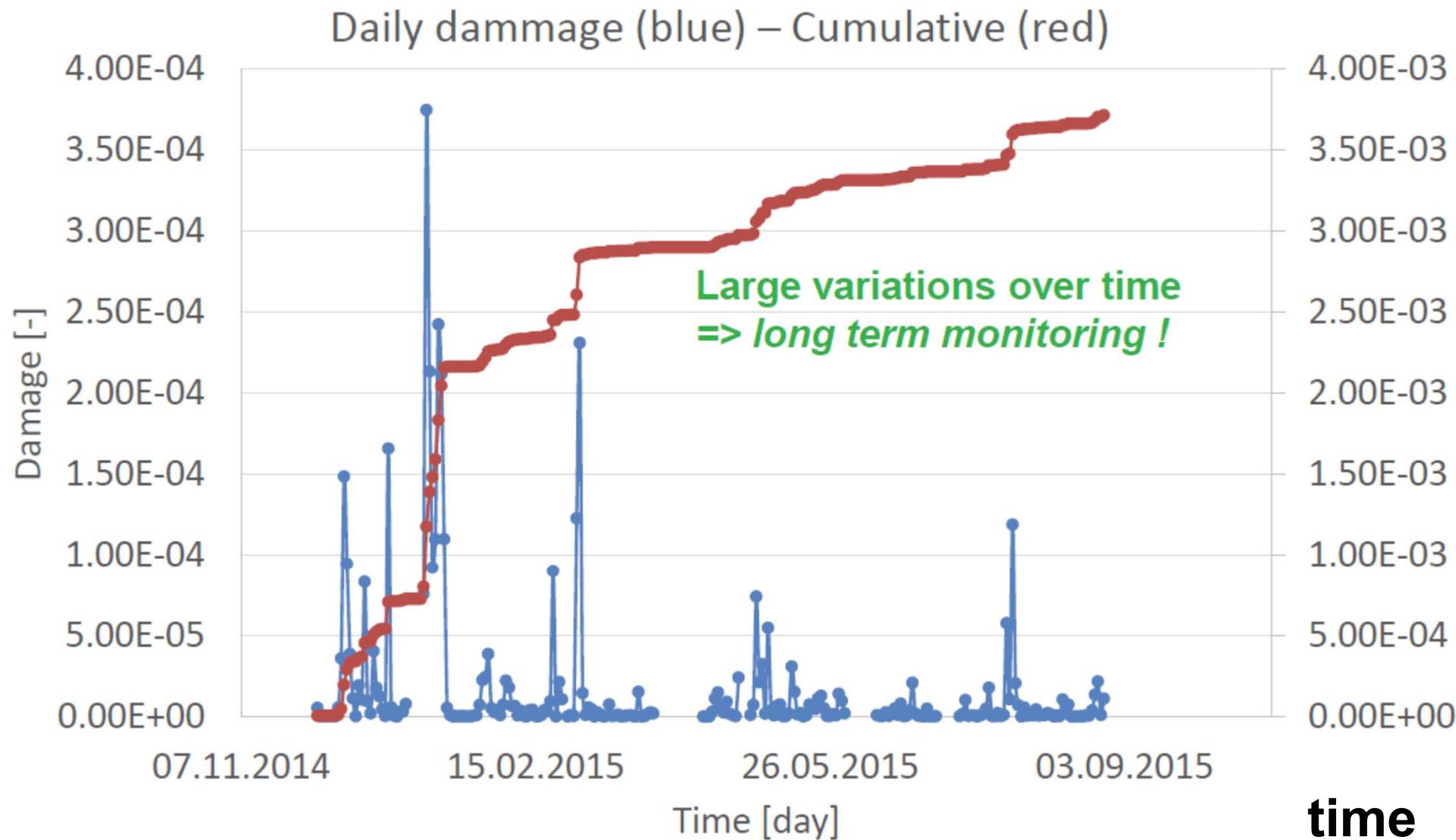
# hub force



**wind speed**

[Loraux, Brühwiler, 2016]

# Fatigue damage : welded joint in the transverse direction



→ fatigue duration of the welded joint is more than an order of magnitude longer than the 20 years indicated by the fabricator

# Case Study

Generalized Pareto Distribution for reliability of bridges exposed to fatigue

M. Nesterova

*Université Paris-Est, SDOA, MAST, IFSTTAR, Marne-la-Vallée, France*

F. Schmidt

*Université Paris-Est, SDOA, MAST, IFSTTAR, Marne-la-Vallée, France*

E. Brühwiler

*MCS, EPFL, Lausanne, Switzerland*

C. Soize

*Université Paris-Est, MSME UMR 8208, Marne-la-Vallée, France*

Millau viaduct,  
France

orthotropic deck

WIM data



Cret de l'Anneau,  
Switzerland

RC slab + steel girders

strains monitoring



# Fatigue safety verification of the BLS Railway Bridge over the Kander River near Wimmis, 1897



Fatigue safety verification of a steel railway bridge using short term monitoring data

B. Sawicki & E. Brühwiler

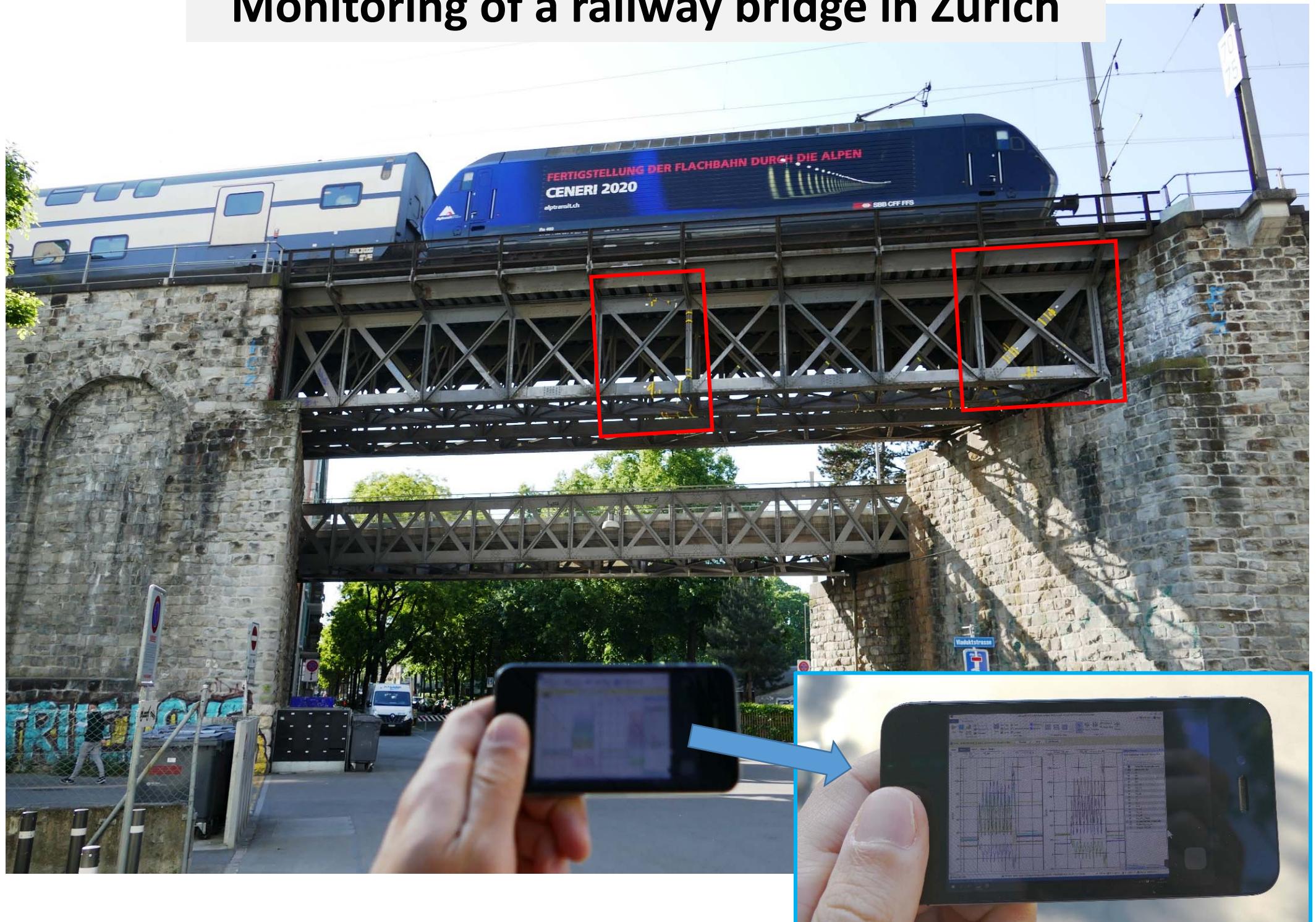
Laboratory of Maintenance and Safety of Structures (MCS), École Polytechnique Fédérale de Lausanne (EPFL), 1015, Lausanne, Switzerland

M. Nesterova

Laboratoire Sécurité et Durabilité des Ouvrages d'Art (SDOA), The French institute of science and technology for transport, spatial planning, development and networks (Ifsttar), 77420, Champs-sur-Marne, France

Photo: 15 April 2017

# Monitoring of a railway bridge in Zurich



# Conclusions

Novel methodology for (fatigue) safety verification is presented :

- ✓ explicit consideration of data from long term monitoring for accurate determination of (fatigue) relevant stresses in structural elements
- ✓ **There is “basically” no fatigue damage** ... in the monitored structural elements.
- ✓ Extreme action events are predicted using monitored data and Extreme Value Theories.
- ✓ Approach is economic.
  
- ✓ Traditional “**Re-calculation**” using “assessment” codes should be abandoned.



# Thank you !

